1. Description
   1. Type This work shall consist of furnishing and constructing a BEBO® bridge system in accordance with these specifications and in reasonably close conformity with the lines, grades, design and dimensions shown on the plans or as established by the Engineer. In situations where two or more specifications apply to this work, the most stringent requirements shall govern.
   2. Designation Precast reinforced concrete BEBO® bridge units manufactured in accordance with this specification shall be designated by span and rise. Precast reinforced concrete wingwalls and headwalls manufactured in accordance with this specification shall be designated by length, height, and deflection angle.
2. Design
   1. Specifications The precast elements are designed in accordance with the "AASHTO LRFD Bridge Design Specifications" 9th Edition, adopted by the American Association of State Highway and Transportation Officials, 2020. A minimum of one and one-half feet of cover above the crown of the bridge units is required in the installed condition. (Unless noted otherwise on the shop drawings and designed accordingly.)
3. Materials
   1. Concrete The concrete for the precast elements shall be air-entrained when installed in areas subject to freeze-thaw conditions, composed of Portland cement, fine and coarse aggregates, admixtures and water. Air-entrained concrete shall contain 6 ± 2 percent air. The air-entraining admixture shall conform to AASHT0 M154. The minimum concrete compressive strength shall be as shown on the shop drawings.
      1. Portland Cement - Shall conform to the requirements of ASTM Specifications C150-Type I, Type II, or Type lIl cement.
      2. Coarse Aggregate - Shall consist of stone having a maximum size of 1 inch. Aggregate shall meet requirements for ASTM C33.
      3. Water Reducing Admixture - The manufacturer may submit, for approval by the Engineer, a water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the concrete.
      4. Calcium Chloride - The addition to the mix of calcium chloride or admixtures containing calcium chloride will not be permitted.
      5. Mixture The aggregates, cement and water shall be proportioned and mixed in a batch mixer to produce a homogeneous concrete meeting the strength requirements of this specification. The proportion of Portland cement in the mixture shall not be less than 564 pounds (6 sacks) per cubic yard of concrete.
   2. Steel Reinforcement
      1. The minimum steel yield strength shall be 60,000 psi, unless otherwise noted on the shop drawings.
      2. All reinforcing steel for the precast elements shall be fabricated and placed in accordance with the detailed shop drawings submitted by the manufacturer.
      3. Reinforcement shall consist of welded wire reinforcing conforming to ASTM Specification A 1064, or deformed billet steel bars conforming to ASTM Specification A 615, Grade 60. Longitudinal distribution reinforcement may consist of welded wire fabric or deformed billet-steel bars.
   3. Steel Hardware
      1. Bolts and threaded rods for wingwall connections shall conform to ASTM A 307. Nuts shall conform to AASHTO M292 (ASTM A194) Grade 2H. All bolts, threaded rods and nuts used in wingwall connections shall be mechanically zinc coated in accordance with ASTM B695 Class 50.
      2. Structural Steel for wingwall connection plates and plate washers shall conform to AASHTO M270 (ASTM A709) Grade 36 and shall be hot dip galvanized as per AASHTO M111 (ASTM A123).
      3. Inserts for wingwalls shall be 1" diameter Two-Bolt Preset Wingwall Anchors as manufactured by Dayton Superior Concrete Accessories, Miamisburg, Ohio, (800) 745-3700 and shall be electro zinc coated in accordance with ASTM B633 SC-10.
      4. Ferrule Loop Inserts shall be F-64 Ferrule Loop Inserts as manufactured by Dayton Superior Concrete Accessories, Miamisburg, Ohio, (800) 745-3700.
      5. Hook Bolts used in attached headwall connections shall be ASTM A307.
      6. Inserts for detached headwall connections shall be AISI Type 304 stainless steel, F-58 Expanded Coil inserts as manufactured by Dayton Superior Concrete Accessories, Miamisburg, Ohio, (800) 745-3700. Coil rods and nuts used in headwall connections shall be AISI Type 304 stainless steel. Washers used in headwall connections shall be either AISI Type 304 stainless steel plate washers or AASHTO M270 (ASTM A709) Grade 36 plate washers hot dip galvanized as per AASHTO M111 (ASTM A123).
      7. Reinforcing bar splices shall be made using the Dowel Bar Splicer System as manufactured by Dayton Superior Concrete Accessories, Miamisburg, Ohio, (800) 745-3700, and shall consist of the Dowel Bar Splicer (DB-SAE) and Dowel-In (DI), or as manufactured by Barsplice Products Inc, Dayton, Ohio, (937) -275-8700, and shall consist of Barsplice XP Type 2 System.
4. Manufacture of Precast Elements Subject to the provisions of Section 5, below, the precast element dimension and reinforcement details shall be as prescribed in the plan and shop drawings provided by the manufacturer.
   1. Forms the forms used in manufacture shall be sufficiently rigid and accurate to maintain the required precast element dimensions within the permissible variations given in Section 5 of these specifications. All casting surfaces shall be of a smooth material.
   2. Placement of Reinforcement
      1. Placement of Reinforcement in Precast Bridge Units - The cover of concrete over the outside circumferential reinforcement shall be 2 inches minimum. The cover of concrete over the inside circumferential reinforcement shall be 1 1/2 inches minimum, unless otherwise noted on the shop drawings. The clear distance of the end circumferential wires shall not be less than one inch nor more than two inches from the ends of each section. Reinforcement shall be assembled utilizing single or multiple layers of welded wire fabric (not to exceed 3 layers), supplemented with a single layer of deformed billet-steel bars, when necessary. Welded wire fabric shall be composed of circumferential and longitudinal wires meeting the spacing requirements of 4.3, below, and shall contain sufficient longitudinal wires extending through the bridge unit to maintain the shape and position of the reinforcement. Longitudinal distribution reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 4.3, below. The ends of the longitudinal distribution reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of the bridge unit.
      2. Placement of Reinforcement for Precast Wingwalls and Headwalls - The cover of concrete over the longitudinal and transverse reinforcement shall be 2 inches minimum. The clear distance from the end of each precast element to the end of reinforcing steel shall not be less than ½ inch nor more than 3 inches. Reinforcement shall be assembled utilizing a single layer of welded wire fabric, or a single layer of deformed billet-steel bars. Welded wire fabric shall be composed of transverse and longitudinal wires meeting the spacing requirements of 4.3, below, and shall contain sufficient longitudinal wires extending through the element to maintain the shape and position of the reinforcement. Longitudinal reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 4.3, below.
   3. Laps, Welds, Spacing
      1. Laps, Welds, and Spacing for Precast Bridge Units - Tension splices in the circumferential reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 5.11.6.1. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 5.11.6.2. The overlap of welded wire fabric shall be measured between the outer-most longitudinal wires of each fabric sheet. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 5.11.5.3. For splices other than tension splices, the overlap shall be a minimum of 12" for welded wire fabric or deformed billet-steel bars. The spacing center to center of the circumferential wires in a wire fabric sheet shall be not less than 2 inches nor more than 4 inches. The spacing center to center of the longitudinal wires shall not be more than 8 inches. The spacing center to center of the longitudinal distribution steel for either line of reinforcing in the top slab shall be not more than 16 inches.
      2. Laps, Welds, and Spacing for Precast Wingwalls and Headwalls - Splices in the reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 5.11.6.1. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 5.11.6.2. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 5.11.5.3. The spacing center-to-center of the wires in a wire fabric sheet shall be not less than 2 inches nor more than 8 inches.
   4. Curing The precast concrete elements shall be cured for a sufficient length of time so that the concrete will develop the specified compressive strength in 28 days or less. Any one of the following methods of curing or combinations thereof shall be used:
      1. Steam Curing - The precast elements may be low-pressure steam cured by a system that will maintain a moist atmosphere.
      2. Water Curing - The precast elements may be water cured by any method that will keep the sections moist.
      3. Membrane Curing - A sealing membrane conforming to the requirements of ASTM Specification C 309 may be applied and shall be left intact until the required concrete compressive strength is attained. The concrete temperature at the time of application shall be within +/- 10 degrees F of the atmospheric temperature. All surfaces shall be kept moist prior to the application of the compounds and shall be damp when the compound is applied.
   5. Storage, Handling & Delivery
      1. Storage

Precast concrete bridge elements shall be lifted and stored in “as-cast” position.

Precast concrete headwall and wingwall units are cast, stored and shipped in a flat position.

The precast elements shall be stored in such a manner to prevent cracking or damage. Store elements using timber supports as appropriate. The units shall not be moved until the concrete compressive strength has reached a minimum of 2500 psi, and they shall not be stored in an upright position.

* + 1. Handling

Handling devices shall be permitted in each precast element for the purpose of handling and setting.

Spreader beams may be required for the lifting of precast concrete bridge elements to preclude damage from bending or torsion forces.

The contractor must provide a double-drum crane with equal capacity on each drum for the installation of the precast elements.

* + 1. Delivery

Precast concrete elements must not be shipped until the concrete has attained the specified design compressive strength, or as directed by the design Engineer.

Precast concrete elements may be unloaded and placed on the ground at the site until installed. Store elements using timber supports as appropriate.

* 1. Quality Assurance The Precaster shall demonstrate adherence to the standards set forth in the NPCA Quality Control Manual. The Precaster shall meet either Section 4.6.1 or 4.6.2
     1. Certification: The Precaster shall be certified by the Precast/Prestressed Concrete Institute Plant Certification Program or the National Precast Concrete Association's Plant Certification Program prior to and during production of the products covered by this specification.
     2. Qualifications, Testing and Inspection
        1. The Precaster shall have been in the business of producing precast concrete products similar to those specified for a minimum of three years. He shall maintain a permanent quality control department or retain an independent testing agency on a continuing basis. The agency shall issue a report, certified by a licensed engineer, detailing the ability of the Precaster to produce quality products consistent with industry standards.
        2. The Precaster shall show that the following tests are performed in accordance with the ASTM standards indicated. Tests shall be performed as indicated in Section 6 of these specifications.
           1. Air Content: C231 or C173
           2. Compressive Strength: C31, C39, C497
        3. The Precaster shall provide documentation demonstrating compliance with this section to CONTECH® Engineered Solutions at regular intervals or upon request.
        4. The Owner may place an inspector in the plant when the products covered by this specification are being manufactured.
     3. Documentation The Precaster shall submit Precast Production Reports to CONTECH® Engineered Solutions as required.

1. Permissible Variations
   1. Bridge Units
      1. Internal Dimensions - The internal dimension shall vary not more than 1% from the design dimensions nor more than 1-1/2 inches whichever is less.
      2. Slab and Wall Thickness - The slab and wall thickness shall not be less than that shown in the design by more than 1/4 inch. A thickness more than that required in the design shall not be cause for rejection.
      3. Length of Opposite Surfaces - Variations in laying lengths of two opposite surfaces of the bridge unit shall not be more than 1/2 inch in any section, except where beveled ends for laying of curves are specified by the purchaser.
      4. Length of Section - The underrun in length of a section shall not be more than 1/2 inch in any bridge unit.
      5. Position of Reinforcement - The maximum variation in position of the reinforcement shall be ± 1/2 inch. In no case shall the cover over the reinforcement be less than 1 1/2 inches for the outside circumferential steel or be less than 1 inch for the inside circumferential steel as measured to the external or internal surface of the bridge. These tolerances or cover requirements do not apply to mating surfaces of the joints.
      6. Area of Reinforcement - The areas of steel reinforcement shall be the design steel areas as shown in the manufacturer's shop drawings. Steel areas greater than those required shall not be cause for rejection. The permissible variation in diameter of any reinforcement shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcement.
   2. Wingwalls & Headwalls
      1. Wall Thickness - The wall thickness shall not vary from that shown in the design by more than 1/2 inch.
      2. Length/ Height of Wall sections - The length and height of the wall shall not vary from that shown in the design by more than 1/2 inch.
      3. Position of Reinforcement - The maximum variation in the position of the reinforcement shall be ± 1/2 inch. In no case shall the cover over the reinforcement be less than 1 1/2 inches.
      4. Size of Reinforcement - The permissible variation in diameter of any reinforcing shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcing. Steel area greater than that required shall not be cause for rejection.
2. Testing/ Inspection
   1. Testing
      1. Type of Test Specimen - Concrete compressive strength shall be determined from compression tests made on cylinders or cores. For cylinder testing, a minimum of 4 cylinders shall be taken for each bridge element. For core testing, a minimum of 2 cores shall be taken for each bridge element. Each element shall be considered separately for the purpose of testing and acceptance
      2. Compression Testing - Cylinders shall be made and tested as prescribed by the ASTM C 39 Specification. Cylinders shall be cured in the same environment as the bridge elements. Cores shall be obtained and tested for compressive strength from each element in accordance with the provisions of the ASTM C42 specification.
      3. Acceptability of Cylinder Tests - When the average compressive strength of all cylinders tested of the same age is equal to or greater than the design compressive strength, and no cylinder tested has a compressive strength less than 90% of required concrete strength, then the element shall be accepted. When the compressive strength of the cylinders tested does not conform to these acceptance criteria, the acceptability of the element may be determined as described in section 6.1.4, below.
      4. Acceptability of Core Tests - The compressive strength of the concrete in a bridge element is acceptable when each core test strength is equal to or greater than the design concrete strength. When the compressive strength of a core tested is less than the design concrete strength, the precast element from which that core was taken may be re-cored. When the compressive strength of the re-core is equal to or greater than the design concrete strength, the compressive strength of the concrete in that bridge element is acceptable.
         1. When the compressive strength of any recore is less than the design concrete strength, the precast element from which that core was taken shall be rejected.
         2. Plugging Core Holes - The core holes shall be plugged and sealed by the manufacturer in a manner such that the elements will meet all of the test requirements of this specification. Precast elements so sealed shall be considered satisfactory for use.
         3. Test Equipment - Every manufacturer furnishing precast elements under this specification shall furnish all facilities and personnel necessary to carryout the test required.
   2. Inspection- The quality of materials, the process of manufacture, and the finished precast elements shall be subject to inspection by the purchaser.
3. Joints The bridge units shall be produced with flat butt ends. The ends of the bridge units shall be such that when the sections are laid together they will make a continuous line with a smooth interior free of appreciable irregularities, all compatible with the permissible variations in section 5, above. The joint width between adjacent precast units shall not exceed 3/4 inches.
4. Workmanship/ Finish The bridge units, wingwalls, and headwalls shall be substantially free of fractures. The ends of the bridge units shall be normal to the walls and centerline of the bridge section, within the limits of the variations given in section 5, above, except where beveled ends are specified. The faces of the wingwalls and headwalls shall be parallel to each other, within the limits of variations given in section 5, above. The surface of the precast elements shall be a smooth steel form or troweled surface. Trapped air pockets causing surface defects shall be considered as part of a smooth, steel form finish.
5. Repairs Precast elements may be repaired, if necessary, because of imperfections in manufacture or handling damage and will be acceptable if, in the opinion of the purchaser, the repairs are sound, properly finished and cured, and the repaired section conforms to the requirements of this specification.
6. Rejection The precast elements shall be subject to rejection on account of any of the specification requirements. Individual precast elements may be rejected because of any of the following:
   1. Fractures or cracks passing through the wall, except for a single end crack that does not exceed one half the thickness of the wall.
   2. Defects that indicate proportioning, mixing, and molding not in compliance with section 4 of these specifications.
   3. Honeycombed or open texture.
   4. Damaged ends, where such damage would prevent making a satisfactory joint.
7. Marking Each bridge unit shall be clearly marked by waterproof paint. The following shall be shown on the inside of the vertical leg of the bridge section:

Bridge Span X Bridge Rise

Date of Manufacture

Name or trademark of the manufacturer

1. Installation Preparation To ensure correct installation of the precast concrete bridge system, care and caution must be exercised in forming the support areas for bridge units, headwall, and wingwall elements. Exercising special care will facilitate the rapid installation of the precast components.
   1. Footings

Do not over excavate foundations unless directed by site soil engineer to remove unsuitable soil.

The site soils engineer shall certify that the bearing capacity meets or exceeds the footing design requirements, prior to the contractor pouring of the footings.

The bridge units and wingwalls shall be installed on either precast or cast-in-place concrete footings. The size and elevation of the footings shall be as designed by the Engineer. A keyway shall be formed in the top surface of the bridge footing as specified on the plans. No keyway is required in the wingwall footings, unless otherwise specified on the plans.

The footings shall be given a smooth float finish and shall reach a compressive strength of 3,000 psi before placement of the bridge and wingwall elements. Backfilling shall not begin until the footing has reached the full design compressive strength.

The footing surface shall be constructed in accordance with grades shown on the plans. When tested with a 10-foot straight edge, the surface shall not vary more than 1/4 inch in 10 feet.

If a precast concrete footing is used, the contractor shall prepare a 4-inch thick base layer of compacted granular material the full width of the footing prior to placing the precast footing.

The foundations for precast concrete bridge elements and wingwalls must be connected by reinforcement to form one monolithic body. Expansion joints shall not be used.

The contractor shall be responsible for the construction of the foundations per the plans and specifications.

1. Installation
   1. General The installation of the precast concrete elements shall be as laid out in the project’s Preconstruction Notes.
      1. Lifting It is the responsibility of the contractor to ensure that a crane of the correct lifting capacity is available to handle the precast concrete units. This can be accomplished by using the weights given for the precast concrete components and by determining the lifting reach for each crane unit. Site conditions must be checked well in advance of shipping to ensure proper crane location and to avoid any lifting restrictions. The lift anchors or holes provided in each unit are the only means to be used to lift the elements. The precast concrete elements must not be supported or raised by other means than those given in the manuals and drawings without written approval from CONTECH® Engineered Solutions.
      2. Construction equipment weight restrictions: In no case shall equipment operating in excess of the design load (HL93) be permitted over the bridge units unless approved by CONTECH® Engineered Solutions.
         1. In the immediate area of the bridge unit, the following restrictions for the use of heavy construction machinery during backfilling operations apply:

* No construction equipment shall cross the bare precast concrete bridge unit.
* After the compacted fill level has reached a minimum of 4 inches over the crown of the bridge, construction equipment with a weight of less than 10 tons may cross the bridge.
* After the compacted fill level has reached a minimum of 1 foot over the crown of the bridge, construction equipment with a weight of less than 30 tons may cross the bridge.
* After the compacted fill level has reached the design cover, or 2 feet, minimum, over the crown of the precast concrete bridge, construction equipment within the design load limits for the road may cross the precast concrete bridge.
  1. Leveling Pad/ Shims The bridge units and wingwalls shall be set on hardboard shims conforming to ASTM D1037 or plastic shims (Dayton Superior P-80, P-81 or approved equal) measuring 5" x 5", minimum, unless shown otherwise on the plans. A minimum gap of 1/2 inch shall be provided between the footing and the bottom of the bridge's vertical legs or the bottom of the wingwall. Also, a supply of 1/4 inch, 1/2 inch & 1/8 inch thick hardboard or plastic shims for various shimming purposes shall be on site.
  2. Placement of Bridge Units

The bridge units shall be placed as shown on the Engineer's plan drawings. Special care shall be taken in setting the elements to the true line and grade. The joint width between adjacent precast units shall not exceed 3/4 inches.

It is the contractor’s responsibility to maintain the structure span during all phases of installation. Due to the arch shape, bridge elements will tend to spread under self-weight. It is imperative that any lateral spreading of the bridge elements be avoided during and after their placement. Therefore, a sufficient quantity of hardwood wedges must be available and on site. The hardwood wedges are placed in the key and smaller shims and wedges added before complete release of the precast concrete bridge element from the crane. Also, a supply of ¼, ½ inch and 1/8-thick hardboard or plastic shims for various shimming purposes should be on site, per section 13.2.

13.3.1. BEBO precast concrete twin-leaf arch units are transported and lifted /rotated in a similar manner as the single-leaf elements. Two double-drum cranes (or one crane and displaceable scaffolding) are required for the erection of the precast concrete arch units.

13.3.2. Ideally, one crane shall be located on each (outer) side of the foundations to independently lift half-arch units from the delivery trucks and into position. The two twin precast concrete units are lifted and positioned simultaneously.

13.3.3. Alternatively, if cranes are to be positioned on the same side of the foundations or within the arch span, they should be located so that the final jointing movement of the units at the crown can be effected without damage to the interlocking joint key.

13.3.4. Before releasing the load of each precast concrete arch half unit from the crane, both elements must be blocked at the foundation key in the correct position and the curved tie rod must be inserted and fixed in the blockouts at the crown joint.

13.3.5. Check the span width at regular intervals to minimize the spreading.

13.3.6. Once correctly positioned and aligned, the precast concrete twin-leaf units are jointed at the crown with cast-in-place concrete as shown in the drawings.

* 1. Placement of Wingwalls & Headwalls

Placement of the Wingwalls and Headwalls - The wingwalls and headwalls shall be placed as shown on the plan drawings. Special care shall be taken in setting the elements to the true line and grade.

* 1. Waterproofing/ Joint protection and Subsurface Drainage
     1. External Protection of Joints - The butt joint made by two adjoining bridge units shall be covered with a 7/8" x 1 3/8" preformed bituminous joint sealant and a minimum of a 9-inch wide joint wrap. The surface shall be free of dirt before applying the joint material. A primer compatible with the joint wrap to be used shall be applied for a minimum width of nine inches on each side of the joint. The external wrap shall be either EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC. or approved equal. The joint shall be covered continuously from the bottom of one bridge section leg, across the top of the bridge and to the opposite bridge section leg. Any laps that result in the joint wrap shall be a minimum of six inches long with the overlap running downhill.
     2. In addition to the joints between bridge units, the joint between the end bridge unit and the headwall shall also be sealed as described above. If precast wingwalls are used, the joint between the end bridge unit and the wingwall shall be sealed with a 2'-0" strip of filter fabric. Also, if lift holes are formed in the bridge units, they shall be primed and covered with a 9" x 9" square of joint wrap.
     3. Crown Joint Waterproofing Membrane- The cast-in-place crown joints connecting two arch leafs shall be covered with waterproofing membrane. The membrane shall be a minimum of 3’-0” wide and overlapped as required per the membrane manufacturer’s recommendations to provide continuous coverage of the arch crown. The surface shall be free of dirt before applying the membrane. A primer compatible with the membrane to be used shall be applied. The membrane shall be bituthene 3000 by W.R. Grace or approved equal. The crown joint shall be continuously covered plus 3’-0” minimum beyond the C.I.P. concrete. Any splices that result in the membrane shall have a minimum of a 6” long lap and with the overlap running downhill. A liquid membrane termination shall be applied to the perimeter of the membrane. Crown joint waterproofing is not required if full arch membrane waterproofing is required.
     4. During the backfilling operation, care shall be taken to keep the joint wrap in its proper location over the joint.
     5. Subsoil drainage shall be as directed by the engineer.
  2. Grouting
     1. Grouting shall not be performed when temperatures are expected to go below 35° for a period of 72 hours. Grouting should be completed as soon as practical after precast arches have been installed.
     2. Fill the bridge-foundation keyway with cement grout (Portland cement and water or cement mortar composed of Portland cement, sand and water) with a minimum 28-day compressive strength of

3000 psi for spans ≤ 48 feet,

5000 psi for spans > 48 feet,

Unless otherwise indicated on the installation drawings. Vibrate as required to ensure that the entire key around the bridge element is completely filled.

* + 1. All grout shall have a maximum aggregate size of ¼ inch.
    2. Lifting and erection anchor recesses shall be filled with grout.
  1. Crown Joint
     1. The crown joint areas must be clean and free of debris before pouring of concrete.
     2. The joints between arch units surrounding the crown joints must be filled so as to not allow wet concrete to seep through joints while the crown joint is being poured.
     3. Concrete used for this crown joint closure pour must have a minimum compressive strength as specified on the drawings.
     4. The concrete for the crown joint shall be air-entrained when installed in areas subject to freeze-thaw conditions, composed of Portland cement, fine and coarse aggregates, admixtures and water. Air-entrained concrete shall contain 6 +/- 2 percent air. The air-entraining admixture shall conform to AASHTO M154.
        1. The Portland cement shall conform to the requirements of ASTM specifications C-150-Type I, Type II, or Type III cement.
        2. The coarse aggregate shall consist of stone having a maximum size of 1 inch. Aggregate shall meet requirements for ASTM C33.
        3. The contractor may submit, for approval by the engineer, a water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the crown joint concrete.
        4. The addition of calcium chloride or admixtures containing calcium chloride will not be permitted.
        5. The aggregates, cement, and water shall be proportioned and mixed to produce a homogeneous concrete meeting the strength requirements of the design.
     5. All reinforcing bars used in the crown joint shall be deformed bars (ASTM A615) grade 60.
     6. 13.7.5.1. Bar reinforcement shall be cut and bent to the shapes shown on the plans. All bars shall be bent cold, unless otherwise permitted.
     7. 13.7.5.2 Bar reinforcement shall be accurately placed as shown on the plans and firmly held in position during the placing and setting of the crown joint concrete. Tack welding of the reinforcement will not be permitted for assembly of reinforcement.
     8. Legs of arches to be fully grouted in foundation keyway before pouring the crown joint. Keyway grout to attain 75% of its design strength (3750 PSI) before crown joint can be poured.
     9. Concrete in crown joint must attain 75% of its design strength before headwall panels can be set on the arch units.
     10. Concrete in crown joint must attain 100% of its design strength before backfilling operations can begin.
     11. Do not pour concrete for the crown joint when temperatures are expected to go below 35° for a period of 72 hours.
     12. No Waterproofing sealant or sealer shall be applied within the crown joint area. Should any areas within the crown joint be covered with sealant or sealer, the areas should be sandblasted to remove the sealant or sealer.
     13. Backfill
     14. Do not perform backfilling during wet or freezing weather.
     15. No backfill shall be placed against any structural elements until they have been approved by the engineer.
     16. Backfill shall be considered as all replaced excavation and new embankment adjacent to the precast concrete elements. The project construction and material specifications, which include the specifications for excavation for structures and roadway excavation and embankment construction, shall apply except as modified in this section.
     17. Backfill Zones
* In-situ soil

Zone A: Constructed embankment or overfill.

* Zone B: Fill that is directly associated with precast concrete bridge installation.

Zone C: Road structure

* + 1. Required Backfill Properties
    2. In-situ soil-Natural ground is to be sufficiently stable to allow effective support to the precast concrete bridge units. As a guide, the existing natural ground should be of similar quality and density to zone b material for minimum lateral dimension of one bridge span outside of the bridge footing.
    3. Zone A-requires fill material with specifications and compacting procedures equal to that for normal road embankments.
    4. Zone B-Generally, soils shall be reasonably free of organic matter, and near concrete surfaces, free of stones larger than 3” in diameter. See charts for detailed descriptions of acceptable soils.
    5. Zone C- Is the road sections of gravel. Asphalt or concrete built in compliance with local engineering practices.
    6. Geotechnical engineer shall review gradations of all interfacing materials and, if necessary, recommend geotextile filter fabric (provided by contractor).
    7. Placing and Compacting backfill- Dumping for backfilling is not allowed any nearer than 3’-0” to a vertical plane through the bridge keyway.

The fill must be placed and compacted in layers not exceeding 8”. The maximum difference in the surface levels of the fill on opposite sides of the bridge must not exceed 2’-0”.

The fill behind wingwalls must be placed at the same time as that of the bridge fill. It must be placed in progressively placed horizontal layers not exceeding 8” per layer.

The backfill of Zone B shall be compacted to a minimum density of 95% of standard proctor as required by AASHTO T-99.

Soil within 1’-0” of concrete surfaces should be hand-compacted. Elsewhere, use of rollers is acceptable. If vibrating roller-compactors are used, they shall not be started or stopped within Zone B and the vibration frequency shall be at least 30 revolutions per second.

The backfill material and compacting behind wingwalls shall satisfy the criteria for the bridge backfill, zone ‘B’.

Backfill against a waterproofed surface shall be placed carefully to avoid damage to the waterproofing material.

* + 1. Bridge Units - For fill heights over 12’-0” (as measured from top crown of bridge to finished grade), no backfilling may begin until a backfill compaction testing plan has been coordinated with and approved by CONTECH® Engineered Solutions.
    2. Wingwalls- Backfill in the front of the wingwalls shall be carried to to ground lines shown in the plans.
    3. Monitoring- The contractor shall check settlements and horizontal displacement of foundation to ensure that they are within the allowable limit provided by the engineer.These measurements should give an indication of the settlements and the deformations along the length of the foundations.

The first measurement row should take place after the erection of all precast bridge system elements. A second after completion of backfilling, and a third before opening the bridge to traffic. Further measurements may be made according to local conditions.





